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Dynamic Effect on Vibration Signatures of Cracks in Railway Prestressed Concrete Sleepers

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Abstract. In general, cracks in structural members indicate a level of serviceability of the engineering structures. Usually a hairline crack can be detected by visual inspection, crack gauge, fibre optical sensor, or laser sensor. Recently, an attempt to develop a non-destructive tool for health monitoring of prestressed concrete sleepers in railway tracks using the vibration responses has been established at the University of Wollongong, Australia. However, the fundamental understanding of the dynamic effect due to the cracks in sleepers is insufficient. This paper investigates the dynamic effect of the cracks on the vibration signatures of the railway prestressed concrete sleepers. The modal analysis has been used to evaluate the modal changes in vibration characteristics of the prestressed concrete sleepers in the frequency band between 0 and 1,600 Hz. The sample cracks have been initiated by the impulsive forces attributed to the high capacity drop impact testing machine. Comparison of the modal parameters of the intact and cracked sleepers has been highlighted in terms of natural frequencies and modal damping. This understanding will lead to further development of the economical technology to evaluate the structural integrity of railway track and its components.

Introduction

Ballasted railway track has become very common in rolling stock transportation and freight, especially in Australian coal and mining businesses. The Australian railway industry spent approximately 25–35 percent of its annual budget on rail track maintenance. The replacement costs of rails and sleepers were a considerable part of the expense [1]. In general, two major parts of ballasted railway track are the super-structure and the sub-structure. The super-structure is made up of steel rails, the fastening systems and sleepers; whilst the ballast, sub-ballast, sub-grade and formation, constitute the sub-structure. Figure 1 illustrates the ballasted track components. Esveld [2] notes that ballasted track has many advantages; for example, the construction costs are comparatively low, the maintenance and repair of track and its components are convenient, it has high damping and very good drainage properties, and noise can be controlled. Since railway track is always subjected to a variety of time-dependent loads, understanding the dynamic track behaviour is essential in order to evaluate the structural safety and service life of the railway track components.

Typically, sleepers are the track components resting on the ballast formation transversely as shown in Figure 1. Railway sleepers were first made in timber, and then a limited number of steel sleepers were used, followed by the now most-popular concrete sleepers. The majority and the trend of modern railway sleepers used worldwide are the prestressed concrete sleepers [3]. Due to the nature of dynamic loadings on railway track, the vibration characteristics of concrete sleepers are essential in analysis and design procedures. Also, to develop and validate a numerical simulation of rail track, the free vibration characteristics of the sleepers in various conditions are needed. Archives of vibration response measurements and parameters of sleepers can help engineers to identify the vibration-based damage or remotely monitor the sleeper health, since it is clear that some sleeper damages incur mostly at resonant frequencies of the sleepers [4]. The resonant

vibrations of sleepers affect not only the sleepers themselves, but also the wheel–rail interaction forces. These effects have been analytically studied by Clark et al. [5], Grassie and Cox [6], and Knothe and Grassie [7]. Due to their wide use, the design and maintenance of prestressed concrete sleepers is a major concern to Australian track engineers.

Fig.1 Typical ballasted track structure

There have been a number of studies related to the evaluation of dynamic properties of concrete sleepers. Experimental modal analysis is one of the widely used techniques to determine the vibration characteristics of concrete sleepers. Ford [8] performed modal analysis on a concrete sleeper in free-free condition using an electrodynamic shaker. Dahlberg and Nielsen [9] developed an analytical model for analysing dynamic behaviour of concrete sleepers in both free-free and in-situ conditions. Based on some experimental results, a two-dimensional dynamic modelling for vibration analysis of concrete sleepers was done by Grassie [4]. It was found that the Timoshenko beam element was the best approximation of the concrete sleepers, even though the elastic properties of prestressed concrete sleepers may not be precise. Recently, Gustavson [10] and Vincent [11] performed the three-dimensional finite element modelling and modal testing of concrete sleepers in free-free condition. The results were in good agreement between numerical and experimental data. In reality, however, the sleepers are placed on ballast/subgrade formation. A comprehensive sleeper-ballast dynamic interaction has rarely been studied. Recently, the studies on rail pad parameters, improper ballast tamping, and the dynamic interaction between ballast and sleeper have been investigated [12-14]. However, the effect on imperfection of ballast and sleeper contact has yet been presented elsewhere.

This paper presents results of experimental modal analysis of both cracked and uncracked prestressed concrete sleepers in laboratory condition. Full-scale prestressed concrete sleepers were provided by an Australian manufacturer, Austrak. Impact hammer excitation technique has been used to evaluate the modal data over the frequency range of interest: 0 to 1,600 Hz. Frequency response functions (FRFs) were obtained using the Bruel&Kjaer PULSE dynamic analyser. The FRFs were processed using the STAR Modal analysis package to identify natural frequencies and the corresponding mode shapes for the sleepers under the particular conditions. The conclusion highlights the effect of cracks on the dynamic properties of prestressed concrete sleepers, in terms of natural frequency change and damping variation. These results will help a development of an alternative method to determine the structural condition of concrete sleepers together with the normal inspection by railway organisations.

Analytical Modal Analysis

Measurements of vibration responses in structures result in the modal parameter identification to obtain the dynamic characteristics of the structures. There are a number of methods to extract the dynamic characteristics, depending on the format of data obtained [15-16].

In a dynamic system, the equation of motion of the system can usually be represented by

$$[M]\{\ddot{x}\} + [C]\{\dot{x}\} + [K]\{x\} = \{f\} \quad (1)$$

where $[M]$ is the mass matrix, $[C]$ is the damping matrix, and $[K]$ is the stiffness matrix. The harmonic force applied to the system with magnitude F and loading frequency ω is given by

$$\{f\} = F \sin \omega t = F e^{j\omega t} \quad (2)$$

The analytical solution can be contrived by Fourier Transform and can be found in [12].

Experimental Modal Analysis

All test specimens were prestressed concrete sleepers designed in accordance with AS1085.14-2003 [17]. The dimensions and masses of the test sleepers are tabulated in Table 1. The excitation points were located on the top surface of the sleeper at every 150 mm along the perimeter [12]. Based on previous experience, it was found that the number of these positions is ample to represent the vibration modes of interest. In this case, an accelerometer had a fixed position whilst an instrumented impact hammer was roved along the excitation points.

Table 1 Dimensions and masses of the test sleepers

Mass (kg)	Gauge Length (m)	Total length (m)	At railseat (m)		At centre (m)	
			width	depth	width	depth
299.5	1.60	2.85	0.22	0.21	0.22	0.18

Figure 3 shows the experimental setup of an uncracked sleeper on full-contact support. The support condition was simulated in laboratory according to AS1085.14 as an alternative material [18]. After performing the modal testing, the fresh sleeper was subjected to a severe impact load of 740kN at both rail seats and then retested. The magnitude of load is associated with the return period of 2,000 years, which is prescribed for ‘*very important*’ track structures [19]. The modal testing was performed again once the cracked sleepers were repeatedly given the impact loads for 50 times. The impact load tests were provided by the high-capacity drop-weight impact machine as shown in Figure 4. The instruments used in this study were a PCB accelerometer, the PCB impact hammer, and the Bruel&Kjaer PULSE vibration analyser. It is important to note that the best position for mounting the accelerometer is at the end of the sleeper. The accelerometer was thus mounted at the sleeper end. Using the hammer to excite vibrations in the sleeper over the frequency range 0 to 1,600 Hz, the 10-time average vibration responses represented by the FRFs were obtained using the PULSE system. Then, processing the recorded FRFs by STARModal, a modal analysis package, gave the natural frequencies and modal damping constants of the sleeper. All procedures were performed twice at each stage.

Results and Discussion

The results of modal testing for intact and cracked sleepers are illustrated in Tables 2. In this table, the first five modes of vibration are presented. For all conditions of the concrete sleepers, it was

found that the first bending mode in a vertical plane clearly dominated the first resonant mode of vibration in either cracked or uncracked conditions. In addition, it is found for the uncracked sleeper that the second frequency is associated with the second bending mode, the third frequency with the lowest torsional mode, the fourth frequency with the third bending mode, and the fifth mode with the fourth bending mode. Similar modes of vibration were identified for the cracked sleepers. Of all five modes, the most significant change in natural frequencies between the uncracked and cracked conditions was the first bending mode. The maximum frequency decrease was about 14 percent and 16 percent for the sleepers after 1 blow and 50 blows of impact, respectively. However, it was found that at higher resonant frequencies, the dynamic effect of cracks was decreased to about 4 to 7 percent difference in the fourth and fifth vibration modes. Also, it should be noted that the cracks played a significant role in increasing the damping values of all flexural vibration modes. The maximum damping increase in the dynamic sleeper-ballast interaction was more than 50 percent in the first bending mode and the average change in other modes was between 10 and 40 percent. The mode shapes of ballasted sleepers were quite difficult to identify in that some mode shapes were sometimes too closely spaced between complex modes of vibration (e.g. Mode 5). Apparently, the cracks tend to have more significant influence on modal results corresponding to the flexural modes of vibration than those associated with the torsional modes. It should be noted that by degrees the dynamic softening phenomenon (natural frequency reduces as the amount of crack increases) prevails when cracks occur.

Fig. 3 Modal testing setup

Fig.4 Impact testing facility

Summary

The experimental results of vibration characteristics of the prestressed concrete sleepers under different levels of structural serviceability are presented in this paper. The dynamic characteristics representing vibration signatures of the sleepers show the trend that cracks in concrete sleepers weaken their flexural stiffness, resulting in the decrease in resonant frequencies. In contrast, it has been evident in the experiments that the cracks develop internal frictions among cement, concrete, and aggregate so that the damping coefficients tend to increase with more crack incurrence.

Table 2 Modal parameters of concrete sleepers with different conditions

Conditions	Mode 1			Mode 2			Mode 3			Mode 4			Mode 5		
	Frequency (Hz)	Dampin g (%)		Frequency (Hz)	Dampin g (%)		Frequency (Hz)	Dampin g (%)		Frequency (Hz)	Dampin g (%)		Frequency (Hz)	Dampin g (%)	
Uncracked sleeper: No crack can be observed.
	177.88	12.68		333.35	3.04		460.06	4.00		616.73	1.85		959.29	1.08	
Sleeper after 1 blow of impact. Only hairline cracks can be observed at rail seats. Crack width is less than 0.2mm
	152.73	16.57		308.25	3.43		438.20	3.61		584.37	2.45		919.26	1.51	
Sleeper after 50 blows of impact. Small cracks can be observed at rail seats. Crack width is about 0.2 to 0.8mm.
	148.78	18.42		304.90	3.63		435.06	3.76		574.80	2.84		899.82	1.49	

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